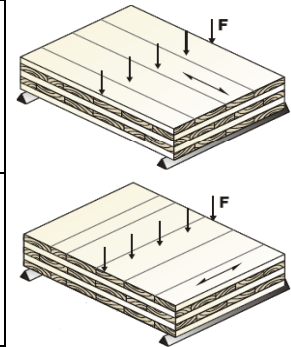


## Design Properties, Nordic Cross-Laminated Timber

### DESIGN PROPERTIES<sup>(1)(2)</sup>

Product	Nordic X-Lam		
Application	Floor and Roof Panels		
Appearance grade	Industrial or Architectural		
Layup Combination	10S-3s	17S-5s	24S-7s
<b>Bending about the longitudinal axis</b>			
Bending moment, $M_0$ (lb-ft/ft)	5325	12166	21418
Shear, $V_0$ (lb/ft)	1485	2475	3465
Compression perp. to grain, $F_{cp}$ (psi) <sup>(3)</sup>	425	425	425
Bending rigidity, $EI_{eff,0}$ ( $10^9$ lbf-in. <sup>2</sup> /ft)	115	438	1078
Shear rigidity, $GA^*_{eff,0}$ ( $10^9$ lbf/ft)	0,39	0,77	1,16
<b>Bending about the transversal axis</b>			
Bending moment, $M_{90}$ (lb-ft/ft)	158	1365	3120
Shear, $V_{90}$ (lb/ft)	1 485	2475	3465
Compression perp. to grain, $F_{cp}$ (psi) <sup>(3)</sup>	425	425	425
Bending rigidity, $EI_{eff,90}$ ( $10^9$ lbf-in. <sup>2</sup> /ft)	3,12	81,1	309
Shear rigidity, $GA^*_{eff,90}$ ( $10^9$ lbf/ft)	0,51	1,02	1,54



<sup>(1)</sup> The tabulated design values are for dry service conditions and normal load duration.

<sup>(2)</sup> Nordic X-Lam bending panels are symmetrical throughout the depth of the member (balanced layups).

<sup>(3)</sup> The shear rigidity values,  $GA^*_{eff}$ , include a shear coefficient form factor,  $\kappa$ , of 1.2.

<sup>(4)</sup> The compression perpendicular to grain values are based on S-P-F No. 3 lumber ( $f_{cp} = 425$  psi).

<sup>(5)</sup> The compression parallel to grain values shall be based on S-P-F MSR 1950f ( $f_c = 1800$  psi) or S-P-F No.3 ( $f_c = 650$  psi) lumber for longitudinal and transversal lamellas, respectively. These values shall be corrected by the column stability factor,  $C_p$ , as defined in NDS-2005.

<sup>(6)</sup> The design properties are based on FPInnovations proposed design method, simplified Kreuzinger. Otherwise, design of cross-laminated timber members shall be in accordance to NDS-2005. (Ref.: *CLT Handbook, Chapter 3 - Structural design of cross-laminated timber elements, FPInnovations, February 2011*)

<sup>(7)</sup> Beams and lintels may be designed based on the simplified method proposed in the CLT Handbook, assuming a composite action between effective longitudinal boards. The design shall be based on the properties of the longitudinal layers in tension and the effective cross-section. (Ref. *CLT Handbook, Chapter 3, Section 2.5.1*)

<sup>(8)</sup> The specific gravity for dowel-type fastener design is 0.42. Member weight shall be based on density of 35 pcf.

\* Nordic X-Lam products have been tested by FPInnovations and are certified by APA.

## Design

### DESIGN

Floor and roof panels	CLT panels are typically designed in single direction, which results in most cases in a conservative solution. The designer must ensure to use an appropriate deflection criteria.
Diaphragms	It is suggested that buildings with CLT diaphragms be designed using the International Building Code analogy (IBC 2009).
Cantilevered elements	For cantilevered CLT slabs, it is suggested that the length, $l$ , be taken as twice the cantilever length, $l_e$ .
Wall panels	Only the layers parallel to the axial load shall be taken into account. For more details, refer to CLT Handbook Chapter 3, Section 2.4.
Lintel design	CLT elements under axial in-plane loads acting as deep beams or lintels may be designed using the capacities shown below and an effective cross-section based on the layers perpendicular to the load. <i>(Ref.: CLT Handbook, Chapter 3, Section 2.5)</i>
DOL and creep	The equation specified in Clause 2.3.2 of NDS-2005 shall be used for calculating the duration of load factor, $C_D$ . The use of a 25% reduction in shear stiffness is recommended when checking the elastic deflection limit and a 50% reduction in shear stiffness for the permanent deformation limit in order to account for the deformations caused by shear perpendicular to grain (rolling shear). These factors have been considered in the selection tables.
Vibration design	The proposed vibration criteria is applicable for: <ol style="list-style-type: none"> <li>1. <u>bare floors</u> with finishes and partitions, without heavy topping;</li> <li>2. vibrations induced by normal walking;</li> <li>3. well supported floors;</li> <li>4. well connected CLT panels;</li> <li>5. inclusion of the self weight of CLT panels only; not live load.</li> </ol>
Fire resistant design	The fire-resistance rating of CLT panels can be calculated using the reduced (or effective) cross-section method and the use of the published design values.

### MATERIAL CHARACTERISTICS

#### Material characteristics

Product	Lamellas	
	Longitudinal	Transversal
Species identification	S-P-F	S-P-F
Stress class	1950f MSR	No.3
Bending at extreme fibre, $F_b$ (psi)	1950	500
Shear parallel to grain, $F_v$ (psi)	135	135
Rolling shear, $F_s$ (psi)	45	45
Compression parallel to grain, $F_c$ (psi)	1800	650
Compression perp. to grain, $F_{cp}$ (psi)	425	425
Tension parallel to grain, $F_t$ (psi)	1375	250
Modulus of elasticity, $E_0$ (psi)	1700000	1200000
Shear modulus, $G_0$ (psi)	106250	75000
Rolling shear modulus, $G_s$ (psi)	10625	7500

#### CLT panel layouts

Product	Composition (L = longitudinal, T = transversal)	Number of plies	Thickness (inch)
105-3s	35L - 35T - 35L	3	4 1/8
175-5s	35L - 35T - 35L - 35T - 35L	5	6 7/8
245-7s	35L - 35T - 35L - 35T - 35L - 35T - 35L	7	9 5/8